

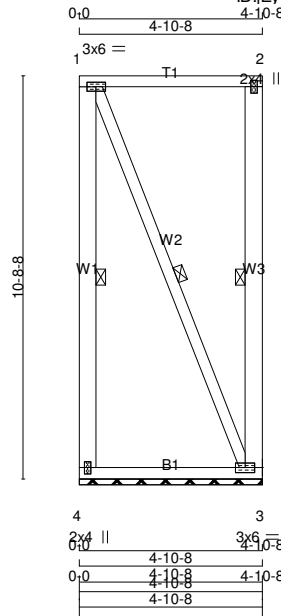
JOB NAME J22-2371-A	TRUSS NAME SPACER01	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Structural Truss Systems, Fort Macleod, Allan VanEssen

Version 8.620 S Dec 12 2022 MITek Industries, Inc. Fri May 19 13:35:18 2023 Page 1

ID: jEyJzokXqZHqOcmWw0e9pzSSGI-ZFFrhbWvp8PsaCP ya5Ym4WtTIGZiD1FWUD8TrzEvS7

Scale = 1:61.2



TOTAL WEIGHT = 60 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
4 - 1	2x6	DRY	No.2	SPF
1 - 2	2x4	DRY	No.2	SPF
3 - 2	2x6	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMW-t	MT20	3.0	6.0		
2	TMV+p	MT20	2.0	4.0		
3	BMW1-t	MT20	3.0	6.0		
4	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
4	197	0	478	-424	-557	4-10-8	4-10-8
3	197	0	432	0	-511	4-10-8	4-10-8

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 557 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 511 LBS FACTORED UPLIFT

PROVIDE FOR 424 LBS FACTORED HORIZONTAL REACTION AT JOINT 4

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	NBCC
4	162	108 / 0	0 / 0	0 / 0	361 / -492	54 / 0	0 / 0
3	162	108 / 0	0 / 0	0 / 0	322 / -453	54 / 0	0 / 0

HORIZONTAL REACTIONS

JT	...	0 / 0	0 / 0	353 / -353	0 / 0	0 / 0
4	...	0 / 0	0 / 0	353 / -353	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 4, 3

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 2x4 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 1-4, 2-3, 1-3. DBS = 20-0-0. CBF = 54 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (10)

MEMB.	C H O R D S			W E B S		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM TO	LENGTH	FR-TO		
4-1	-449 / 573	0.0	0.0 0.38 (7)	6.25	1-3	-500 / 500 0.34 (4)
1-2	-201 / 174	-63.1	-63.1 0.26 (1)	6.25		
3-2	-154 / 121	0.0	0.0 0.30 (6)	6.25		
4-3	-413 / 375	-17.5	-17.5 0.11 (3)	6.25		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.7.1.(7)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 8.4 } PSF AT { 40-0-0 } FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE { MAIN WIND FORCE RESISTING SYSTEM}. INTERNAL WIND PRESSURE IS BASED ON DESIGN { CATEGORY 2 }. BUILDING MAY BE LOCATED ON { OPEN TERRAIN }, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST { 0-0 } FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 4.0 PSF AND 4.0 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 22.2 PSF
DL = 4.0 PSF
BOT CH. LL = 0.0 PSF
NBCC LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR FARM AND LOW HUMAN OCCUPANCY BUILDING REQUIREMENTS OF NFBC 1995

THIS DESIGN COMPLIES WITH:
- CSA 086-01
- TPIC 1996 (LSD)

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED

80 % OF 25.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 22.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.38/1.00 (1-4:7) , BC=0.11/1.00 (3-4:3) , WB=0.34/1.00 (1-3:4) , SSI=0.12/1.00 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 IMPORTANCE FACTOR=0.80

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MT20	618	354	1667 822 2284 1656

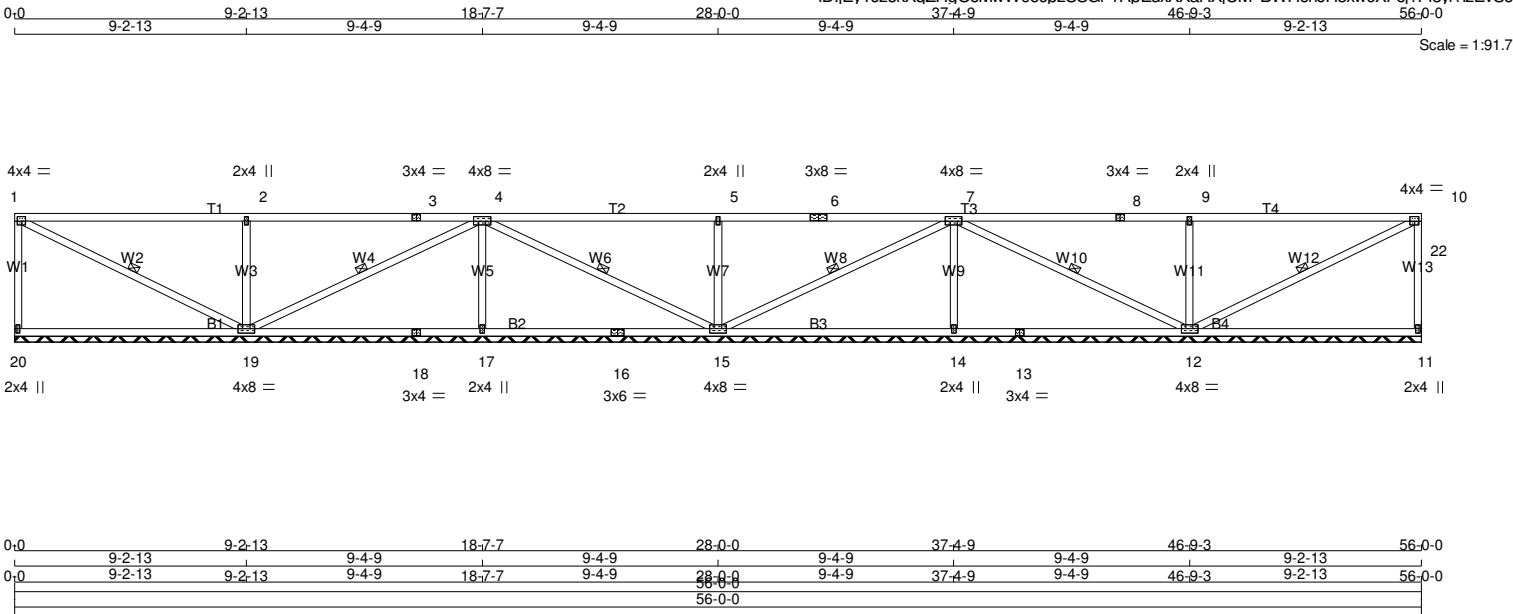
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (1) (INPUT = 0.90)
JSI METAL= 0.21 (4) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
J22-2371-A	SPACER56	1	1	TRUSS DESC.	

Structural Truss Systems, Fort Macleod, Allan VanEssen Version 8.620 S Dec 12 2022 MiTek Industries, Inc. Fri May 19 13:35:19 2023 Page 1



TOTAL WEIGHT = 230 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
20 - 1	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
3 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
20 - 18	2x4	DRY	No.2	SPF
18 - 16	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
13 - 11	2x4	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMW-t	MT20	4.0	4.0		
2, 5, 9						
2	TMW+w	MT20	2.0	4.0		
3	TS-t	MT20	3.0	4.0		
4	TMWWW-t	MT20	4.0	8.0		
6	TS-t	MT20	3.0	8.0		
7	TMWWW-t	MT20	4.0	8.0		
8	TS-t	MT20	3.0	4.0		
10	TMW-t	MT20	4.0	4.0		
11	BMV1+p	MT20	2.0	4.0		
12, 15, 19						
12	BMWWW1-t	MT20	4.0	8.0		
13	BS-t	MT20	3.0	4.0		
14	BMW1+w	MT20	2.0	4.0		
16	BS-t	MT20	3.0	6.0		
17	BMW1+w	MT20	2.0	4.0		
18	BS-t	MT20	3.0	4.0		
20	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT IN-SX	REQRD BRG
	VERT	HORZ	DOWN	UPLIFT		
20	280	0	280	-186	56-0-0 (16-0-00-0-0)	
11	280	0	280	0	56-0-0 (16-0-00-0-0)	
19	893	0	893	0	-516 56-0-0 (16-0-00-0-0)	
17	669	0	669	0	-302 56-0-0 (16-0-00-0-0)	
15	834	0	834	0	-492 56-0-0 (16-0-00-0-0)	
14	669	0	669	0	-303 56-0-0 (16-0-00-0-0)	
12	893	0	893	0	-516 56-0-0 (16-0-00-0-0)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT 20 FOR 164 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 150 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 516 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 302 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 492 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 303 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 516 LBS FACTORED UPLIFT

PROVIDE FOR 186 LBS FACTORED HORIZONTAL REACTION AT JOINT 20

UNFACTORED REACTIONS

JT	COMBINED	MAX / MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	NBCC
20	230	152 / 0	0 / 0	0 / 0	0 / -176	78 / 0	0 / 0
11	230	151 / 0	0 / 0	0 / 0	0 / -151	78 / 0	0 / 0
19	734	497 / 0	0 / 0	0 / 0	0 / -554	237 / 0	0 / 0
17	549	358 / 0	0 / 0	0 / 0	0 / -348	191 / 0	0 / 0
15	686	467 / 0	0 / 0	0 / 0	0 / -524	218 / 0	0 / 0
14	549	358 / 0	0 / 0	0 / 0	0 / -350	191 / 0	0 / 0
12	734	497 / 0	0 / 0	0 / 0	0 / -554	237 / 0	0 / 0

HORIZONTAL REACTIONS

20	---	0 / 0	0 / 0	0 / 0	155 / -155	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 20, 11, 19, 17, 15, 14, 12

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x4 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 1-19, 4-19, 4-15, 7-15, 7-12, 10-12. DBS = 20-0-0. CBF = 9 LBS.

DBS = DIAGONAL BRACE SPACING (MAX), CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (10)

MEMB.	FR-TO	CHORDS			WEBS			
		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LCI MAX (LC)	MAX. FACTORED UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
20-21		-216 / 189	0.0	0.0 0.13 (4)	7.81	1-19	-58 / 76	0.03 (4)
21-1		-216 / 189	0.0	0.0 0.13 (4)	7.81	19-2	-665 / 522	0.17 (1)
1-2		-37 / 30	-63.1	-63.1 0.79 (1)	6.25	19-4	-74 / 119	0.04 (6)
2-3		-37 / 30	-63.1	-63.1 0.79 (1)	6.25	17-4	-510 / 363	0.13 (1)
3-4		-37 / 30	-63.1	-63.1 0.79 (1)	6.25	4-15	-73 / 96	0.04 (1)
4-5		-64 / 33	-63.1	-63.1 0.66 (1)	6.25	15-5	-602 / 473	0.15 (1)
5-6		-64 / 33	-63.1	-63.1 0.66 (1)	6.25	15-7	-73 / 96	0.04 (1)
6-7		-64 / 33	-63.1	-63.1 0.66 (1)	6.25	14-7	-510 / 365	0.13 (1)
7-8		-60 / 30	-63.1	-63.1 0.79 (1)	6.25	7-12	-68 / 99	0.04 (1)
8-9		-60 / 30	-63.1	-63.1 0.79 (1)	6.25	12-9	-665 / 522	0.17 (1)
9-10		-60 / 30	-63.1	-63.1 0.79 (1)	6.25	12-10	-32 / 51	0.02 (6)
11-22		-216 / 158	0.0	0.0 0.13 (8)	7.81			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH LL = 22.2 PSF
 DL = 4.0 PSF
 BOT CH LL = 0.0 PSF
 NBCC LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR FARM AND LOW HUMAN OCCUPANCY BUILDING REQUIREMENTS OF NFBC 1995

THIS DESIGN COMPLIES WITH:
 - CSA 086-01
 - TPIC 1996 (LSD)

DESIGN ASSUMPTIONS
 - SLOPE REDUCTION FACTOR USED

80 % OF 25.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 22.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.79/1.00 (2-4:1) , BC=0.34/1.00 (17-19:3) , WB=0.17/1.00 (2-19:1) , SSI=0.28/1.00 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 IMPORTANCE FACTOR=0.80

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (3) (INPUT = 0.90)

JSI METAL= 0.52 (6) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
J22-2371-A	SPACER56	1	1	TRUSS DESC.	

Structural Truss Systems, Fort Macleod, Allan VanEssen

Version 8.620 S Dec 12 2022 MITek Industries, Inc. Fri May 19 13:35:19 2023 Page 2

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LOADING

TOTAL LOAD CASES: (10)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)
FR-TO		FROM TO			FR-TO		
22-10	-216 / 158	0.0	0.0 0.13 (8)	7.81			
20-19	-182 / 165	-17.5	-17.5 0.33 (3)	6.25			
19-18	-82 / 41	-17.5	-17.5 0.34 (3)	6.25			
18-17	-82 / 41	-17.5	-17.5 0.34 (3)	6.25			
17-16	-82 / 41	-17.5	-17.5 0.28 (3)	6.25			
16-15	-82 / 41	-17.5	-17.5 0.28 (3)	6.25			
15-14	-72 / 34	-17.5	-17.5 0.28 (3)	6.25			
14-13	-72 / 34	-17.5	-17.5 0.34 (3)	6.25			
13-12	-72 / 34	-17.5	-17.5 0.34 (3)	6.25			
12-11	-88 / 76	-17.5	-17.5 0.33 (3)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.7.1.(7)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 8.4} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}. INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 4.0 PSF AND 4.0 PSF RESPECTIVELY.

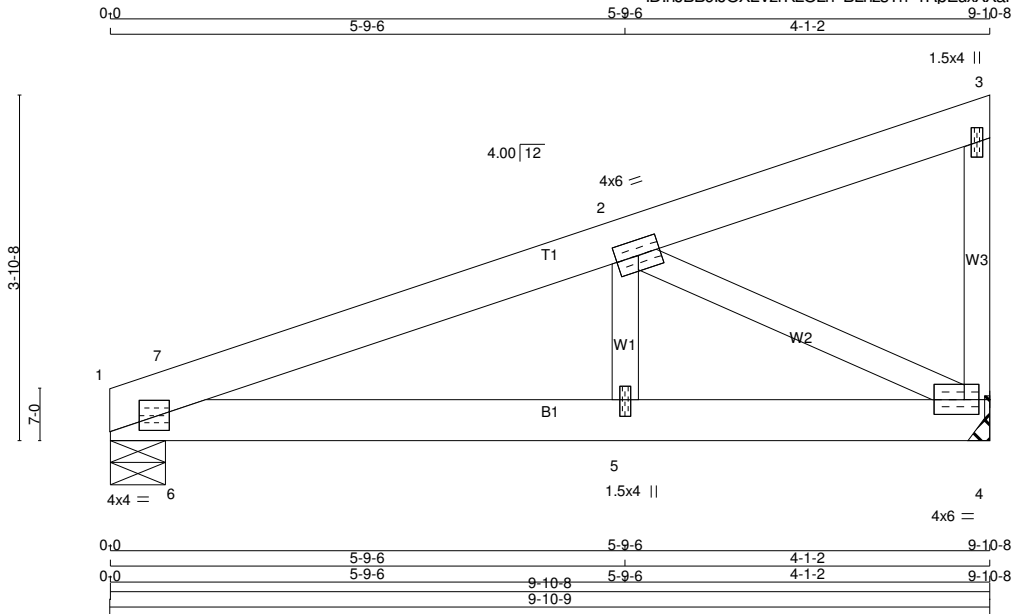
JOB NAME J22-2371-A	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Structural Truss Systems, Fort Macleod, Allan VanEssen

Version 8.620 S Dec 12 2022 MITek Industries, Inc. Fri May 19 13:35:19 2023 Page 1

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9-10-8

Scale = 1:25.9



TOTAL WEIGHT = 46 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
1 - 3	2x6	DRY	No.2
4 - 3	2x4	DRY	No.2
1 - 4	2x6	DRY	No.2

ALL WEBS 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMB1-l	MT20	4.0	4.0	2.00	1.50
2	TMWW-t	MT20	4.0	6.0		
3	TMV+p	MT20	1.5	4.0		
4	BMWV1-t	MT20	4.0	6.0	2.00	2.00
5	BMW+w	MT20	1.5	4.0	2.25	0.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
4	1499	0	1637	0	-762	MECHANICAL
1	1499	0	1637	679	-679	7-7

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT 4. MINIMUM BEARING LENGTH AT JOINT 4 = 3-0.

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 762 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 1 FOR 679 LBS. FACTORED UPLIFT

PROVIDE FOR 679 LBS. FACTORED HORIZONTAL REACTION AT JOINT 1

UNFACTORED REACTIONS

JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
4		1131	680 / 0	247 / 0	0 / 0	0 / -703	296 / 0	0 / 0
1		1131	680 / 0	247 / 0	0 / 0	0 / -644	296 / 0	0 / 0

HORIZONTAL REACTIONS

JT	---	0 / 0	0 / 0	0 / 0	485 / 0	0 / 0	0 / 0
1							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 1

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

MEMB.	C H O R D S				W E B S			
	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO			
1-7	-2555 / 818	-209.8	-209.8	0.15 (11)	5.12	5-2	0 / 697	0.12 (5)
7-2	-2380 / 1005	-209.8	-209.8	0.45 (2)	4.76	2-4	-2561 / 1311	0.59 (2)
2-3	-188 / 132	-209.8	-209.8	0.40 (2)	6.25	6-7	-330 / 479	0.00 (1)
4-3	-355 / 227	0.0	0.0	0.27 (11)	7.81			
1-6	-987 / 2293	-93.8	-93.8	0.52 (2)	6.25			
6-5	-987 / 2293	-93.8	-93.8	0.59 (2)	6.25			
5-4	-987 / 2293	-93.8	-93.8	0.46 (2)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {10.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.8	PSF
	DL = 5.0	PSF
BOT CH.	LL = 10.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 45.8	PSF

SPACING = 60.0 IN.C.C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD)
TIMES IMPORTANCE FACTOR EQUALS 23.8 P.S.F.
SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L / 999 (0.05")
ALLOWABLE DEFL.(TL)= L/180 (0.66")
CALCULATED VERT. DEFL.(TL) = L / 999 (0.07")

CSI: TC=0.45/1.00 (2-7-2), BC=0.59/1.00 (5-6-2),
WB=0.59/1.00 (2-4-2), SSI=0.44/1.00 (2-7-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS= 1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

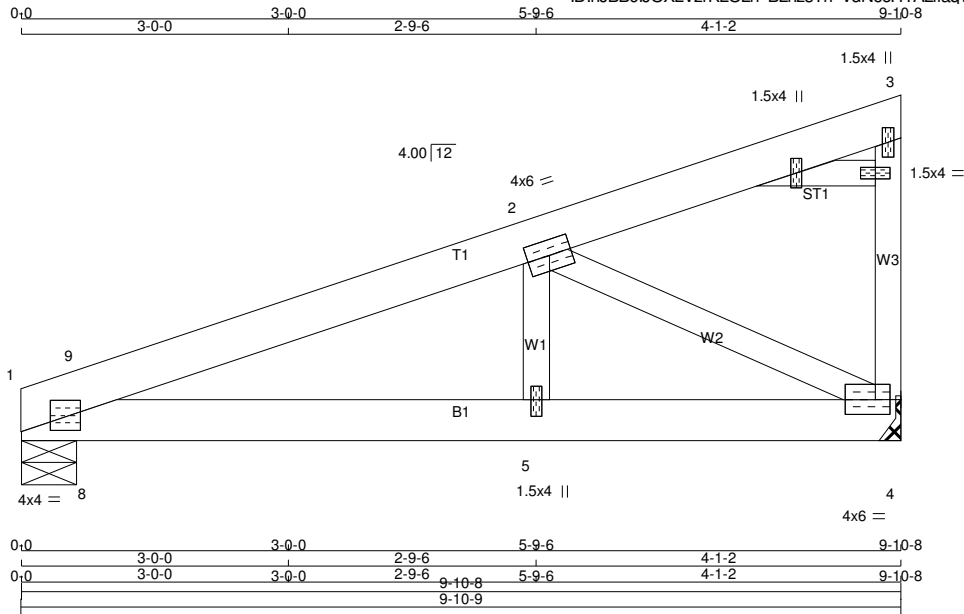
JSI GRIP= 0.88 (1) (INPUT = 0.90)
JSI METAL= 0.93 (1) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
J22-2371-A	T10GE	2	1	TRUSS DESC.	

Structural Truss Systems, Fort Macleod, Allan VanEssen

Version 8.620 S Dec 12 2022 MiTek Industries, Inc. Fri May 19 13:35:20 2023 Page 1

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TOTAL WEIGHT = 2 X 48 = 96 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
1 - 3	2x6	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
1 - 4	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF

ALL GABLE WEBS 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0.0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMB1-1	MT20	4.0	4.0	2.00	1.50
2	TMVW-1	MT20	4.0	6.0		
3	TMV+p	MT20	1.5	4.0		
4	BMVW1-1	MT20	4.0	6.0	2.00	2.00
5	BMV+w	MT20	1.5	4.0	2.25	0.75
6	NP+w	MT20	1.5	4.0		
7	NP+p	MT20	1.5	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
			DOWN	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
4	1499	0	1637	0	1637	0	0/0	0/-703	296/0	0/0
1	1499	0	1637	679	1637	679	-679	7-7	7-7	7-7

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT 4. MINIMUM BEARING LENGTH AT JOINT 4 = 3-0.

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 762 LBS. FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT 1 FOR 679 LBS. FACTORED UPLIFT

PROVIDE FOR 679 LBS. FACTORED HORIZONTAL REACTION AT JOINT 1

UNFACTORED REACTIONS

JT	COMBINED	MAX /MIN COMPONENT REACTIONS						
		1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
4	1131	680/0	247/0	0/0	0/0	0/-703	296/0	0/0
1	1131	680/0	247/0	0/0	0/0	0/-644	296/0	0/0

HORIZONTAL REACTIONS

JT	---	0/0	0/0	0/0	485/0	0/0	0/0
1	---	0/0	0/0	0/0	485/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 1

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 4.76 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (18)

MEMB.	C H O R D S				W E B S			
	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (CSI (LC))	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC LENGTH
FR-TO					FR-TO			
1-9	-2555 / 818	-209.8	-209.8 0.15 (11)	5.12	5-2	0 / 697	0.12 (5)	
9-2	-2380 / 1005	-209.8	-209.8 0.45 (2)	4.76	2-4	-2561 / 1311	0.59 (2)	
2-3	-188 / 132	-209.8	-209.8 0.40 (2)	6.25	8-9	-330 / 479	0.00 (1)	
4-3	-355 / 227	0.0	0.0 0.27 (11)	7.81				
1-8	-987 / 2293	-93.8	-93.8 0.52 (2)	6.25				
8-5	-987 / 2293	-93.8	-93.8 0.59 (2)	6.25				
5-4	-987 / 2293	-93.8	-93.8 0.46 (2)	6.25				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {10.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.8 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 45.8 PSF

SPACING = 60.0 IN.C.C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 4 OF BCBC 2018, NBC-2019AE
 - PART 4 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 - SLOPE REDUCTION FACTOR USED

(80% OF 27.2 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD)
 TIMES IMPORTANCE FACTOR EQUALS 23.8 P.S.F.
 SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
 ALLOWABLE DEFL.(TL)= L/180 (0.66")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.45/1.00 (2-9-2), BC=0.59/1.00 (5-8-2), WB=0.59/1.00 (2-4-2), SSI=0.44/1.00 (2-9-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

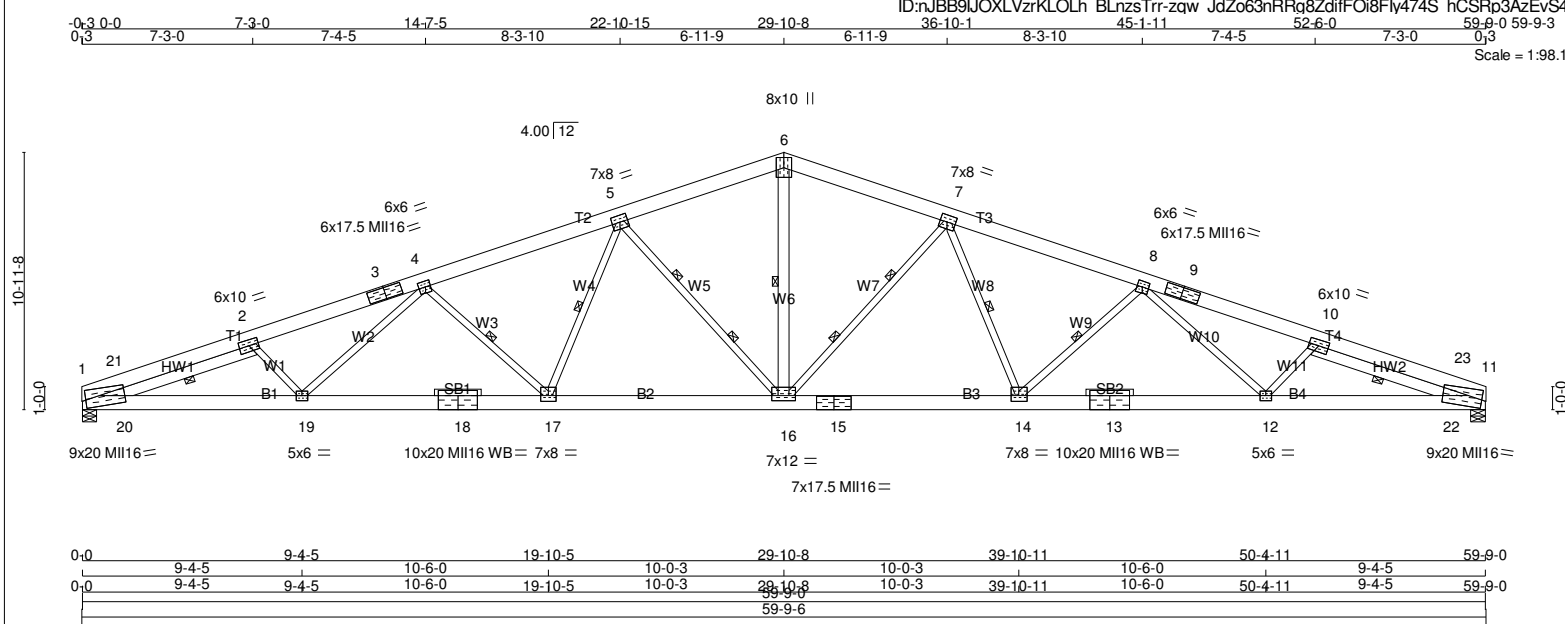
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (1) (INPUT = 0.90)
 JSI METAL= 0.93 (1) (INPUT = 1.00)



TOTAL WEIGHT = 19 X 472 = 8977 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
1 - 3	2x8	DRY	2250F 1.9E
3 - 6	2x8	DRY	2250F 1.9E
6 - 9	2x8	DRY	2250F 1.9E
9 - 11	2x8	DRY	2250F 1.9E
1 - 18	2x8	DRY	2250F 1.9E
18 - 15	2x8	DRY	2250F 1.9E
15 - 13	2x8	DRY	2250F 1.9E
13 - 11	2x8	DRY	2250F 1.9E

REINFORCING MEMBERS

MEMBER	SIZE	DRY	2100F 1.8E	SPF
HW1	2x6	DRY	2100F 1.8E	SPF
HW2	2x6	DRY	2100F 1.8E	SPF

ALL WEBS EXCEPT

MEMBER	SIZE	DRY	2100F 1.8E	SPF
5 - 16	2x4	DRY	2100F 1.8E	SPF
16 - 6	2x6	DRY	2100F 1.8E	SPF
16 - 7	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	UP	IN-SX	IN-SX	IN-SX	IN-SX
1	9069	0	9069	755	-3880	7-7	7-7	
11	9069	0	9069	0	-3880	7-7	7-7	

PROVIDE ANCHORAGE AT BEARING JOINT 1 FOR 3880 LBS. FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 3880 LBS. FACTORED UPLIFT

PROVIDE FOR 755 LBS. FACTORED HORIZONTAL REACTION AT JOINT 1

UNFACTORED REACTIONS

JT	1ST LCASE		MAX / MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
1	6843	3557 / 0	1494 / 0	0 / 0	0 / -3732	1792 / 0	0 / 0
11	6843	3557 / 0	1494 / 0	0 / 0	0 / -3732	1792 / 0	0 / 0

HORIZONTAL REACTIONS

JT	REACT
1	0 / 0

SUPPORT MUST HAVE A MINIMUM 1300 PSI FACTORED BEARING RESISTANCE AT JOINT(S) 1, 11

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 2.24 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 4.19 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
 1 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-17, 5-17, 6-16, 7-14, 8-14, 2-20, 10-22.
 2 LATERAL BRACE(S) AT 1/3 LENGTH OF 5-16, 7-16.

LOADING

TOTAL LOAD CASES: (18)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRAC LENGTH (FR-TO)
FR-TO	FROM TO	FR-TO	FR-TO
1-21	-16662 / 6958	-209.8 -209.8	0.46 (1)
21-2	-15141 / 6625	-209.8 -209.8	0.57 (1)
2-3	-21368 / 9204	-209.8 -209.8	0.92 (1)
3-4	-21368 / 9204	-209.8 -209.8	0.92 (1)
4-5	-18094 / 7800	-209.8 -209.8	0.78 (1)
5-6	-13944 / 6113	-209.8 -209.8	0.53 (1)
6-7	-13944 / 6113	-209.8 -209.8	0.53 (1)
7-8	-18094 / 7801	-209.8 -209.8	0.78 (1)
8-9	-21368 / 9207	-209.8 -209.8	0.92 (1)
9-10	-21368 / 9207	-209.8 -209.8	0.92 (1)
10-23	-15141 / 6640	-209.8 -209.8	0.57 (1)
23-11	-16662 / 6973	-209.8 -209.8	0.46 (1)
1-20	-6508 / 14392	-93.8 -93.8	0.67 (1)
20-19	-9036 / 20511	-93.8 -93.8	0.95 (1)
19-18	-8074 / 19083	-93.8 -93.8	0.87 (1)
18-17	-8074 / 19083	-93.8 -93.8	0.87 (1)
17-16	-6264 / 16068	-93.8 -93.8	0.76 (1)
16-15	-5713 / 16068	-93.8 -93.8	0.76 (1)
15-14	-5713 / 16068	-93.8 -93.8	0.76 (1)
14-13	-7524 / 19083	-93.8 -93.8	0.87 (1)
13-12	-7524 / 19083	-93.8 -93.8	0.87 (1)
12-22	-8489 / 20511	-93.8 -93.8	0.95 (1)
22-11	-5971 / 14392	-93.8 -93.8	0.67 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {10.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}. INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.8 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 45.8 PSF

SPACING = 60.0 IN. C.C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 4 OF BCBC 2018, NBC-2019AE
 - PART 4 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 - SLOPE REDUCTION FACTOR USED

(80% OF 27.2 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD)
 TIMES IMPORTANCE FACTOR EQUALS 23.8 P.S.F.
 SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.99")
 CALCULATED VERT. DEFL.(LL) = L/900 (0.80")
 ALLOWABLE DEFL.(TL) = L/180 (3.98")
 CALCULATED VERT. DEFL.(TL) = L/612 (1.17")

CSI: TC=0.92/1.00 (2-4:1), BC=0.95/1.00 (19-20:1),
 WB=0.84/1.00 (8-14:3), SSI=0.44/1.00 (4-5:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
 SHEAR=1.00 TENS=1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MII16	438	302	2547
MT20	650	371	1747

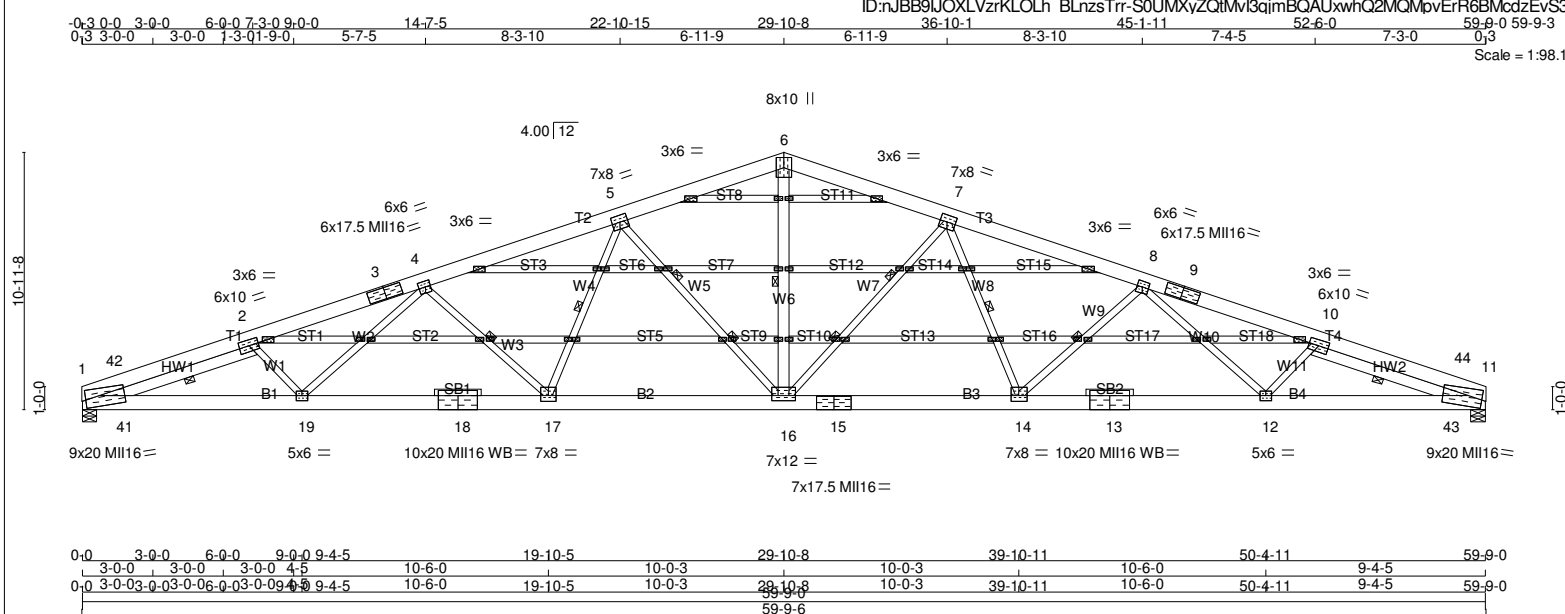
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (6) (INPUT = 0.90)
 JSI METAL= 0.98 (18) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
J22-2371-A	T60GE	2	1	TRUSS DESC.	

Structural Truss Systems, Fort Macleod, Allan VanEssen ID:nJBB9JQXLVzrKLOLh BLnzsTrr-SOUMXyZQitMv3qimBQAUxwhQ2MQMpvErR6MBmdzEvS3 Version 8.620 S Dec 12 2022 MITek Industries, Inc. Fri May 19 13:35:22 2023 Page 1



TOTAL WEIGHT = 2 X 561 = 1121 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
1 - 3	2x8	DRY	2250F 1.9E	DF
3 - 6	2x8	DRY	2250F 1.9E	DF
6 - 9	2x8	DRY	2250F 1.9E	DF
9 - 11	2x8	DRY	2250F 1.9E	DF
1 - 18	2x8	DRY	2250F 1.9E	DF
18 - 15	2x8	DRY	2250F 1.9E	DF
15 - 13	2x8	DRY	2250F 1.9E	DF
13 - 11	2x8	DRY	2250F 1.9E	DF

REINFORCING MEMBERS

HW1	2x6	DRY	2100F 1.8E	SPF
HW2	2x6	DRY	2100F 1.8E	SPF

ALL WEBS EXCEPT

5 - 16	2x4	DRY	2100F 1.8E	SPF
16 - 6	2x6	DRY	2100F 1.8E	SPF
16 - 7	2x4	DRY	2100F 1.8E	SPF

ALL GABLE WEBS

2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMBW1-m	MII16	9.0	20.0	3.75	7.00
2	TMMWW-t	MT20	6.0	10.0		
3	TS-t	MII16	6.0	17.5		
4	TMMWW-t	MT20	6.0	3.00	2.75	
5	TMMWW-t	MT20	7.0	8.0	3.50	3.75
6	TTW+p	MT20	8.0	10.0		
7	TMMWW-t	MT20	7.0	8.0	3.50	3.75
8	TMMWW-t	MT20	6.0	6.0	3.00	2.75
9	TS-t	MII16	6.0	17.5		
10	TMMWW-t	MT20	6.0	10.0		
11	TMBW1-m	MII16	9.0	20.0	3.75	7.00
12	BMWW-t	MT20	5.0	6.0		
13	BS-t	MII16	10.0	20.0		
14	BMWW-t	MT20	7.0	8.0	2.75	4.00
15	BS-t	MII16	7.0	17.5		
16	BMWW-t	MT20	7.0	12.0	2.50	6.00
17	BMWW-t	MT20	7.0	8.0	2.75	4.00
18	BS-t	MII16	10.0	20.0		
19	BMWW-t	MT20	5.0	6.0		
20, 20, 22, 22, 23, 24, 24, 25, 25, 26, 26, 27, 28, 28, 30, 30, 31, 33, 33, 34, 34, 35, 35, 37, 40, 40						
20	NP+w	MT20	2.0	4.0		
21, 29, 32, 36, 38, 39						
21	NP+w	MT20	3.0	6.0		
23, 27, 31, 37						
23	NP+w	MT20	2.0	4.0	1.75	1.00

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
1	9069	0	9069	755	-3880	7-7	7-7
11	9069	0	9069	0	-3880	7-7	7-7

PROVIDE ANCHORAGE AT BEARING JOINT 1 FOR 3880 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 3880 LBS. FACTORED UPLIFT

PROVIDE FOR 755 LBS. FACTORED HORIZONTAL REACTION AT JOINT 1

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
1	6843	3557 / 0	1494 / 0	0 / 0	0 / -3732	1792 / 0	0 / 0
11	6843	3557 / 0	1494 / 0	0 / 0	0 / -3732	1792 / 0	0 / 0

HORIZONTAL REACTIONS

JT	---	0 / 0	0 / 0	539 / -539	0 / 0	0 / 0
1						

SUPPORT MUST HAVE A MINIMUM 1300 PSI FACTORED BEARING RESISTANCE AT JOINT(S) 1, 11

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.19 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

- 1 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-17, 5-17, 6-16, 7-14, 8-14, 2-41, 10-43.
- 2 LATERAL BRACE(S) AT 1/3 LENGTH OF 5-16, 7-16.

LOADING
TOTAL LOAD CASES: (18)

C H O R D S				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LCL1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO					FR-TO			
1-42	-16662 / 6958	-209.8	-209.8	0.46 (1)	2.95	2-19	-834 / 850	0.13 (13)
42-2	-15141 / 6625	-209.8	-209.8	0.57 (1)	2.99	19-4	-687 / 2111	0.51 (13)
2-3	-21368 / 9204	-209.8	-209.8	0.92 (1)	2.24	4-17	-3122 / 1960	0.84 (2)
3-4	-21368 / 9204	-209.8	-209.8	0.92 (1)	2.24	17-5	-1226 / 3411	0.60 (2)
4-5	-18094 / 7800	-209.8	-209.8	0.78 (1)	2.61	5-16	-4923 / 2673	0.80 (2)
5-6	-13944 / 6113	-209.8	-209.8	0.53 (1)	3.15	16-6	-3092 / 7619	0.64 (12)
6-7	-13944 / 6113	-209.8	-209.8	0.53 (1)	3.15	16-7	-4923 / 2674	0.80 (3)
7-8	-18094 / 7801	-209.8	-209.8	0.78 (1)	2.61	7-14	-1226 / 3411	0.60 (3)
8-9	-21368 / 9207	-209.8	-209.8	0.92 (1)	2.24	14-8	-3122 / 1961	0.84 (3)
9-10	-21368 / 9207	-209.8	-209.8	0.92 (1)	2.24	8-12	-691 / 2111	0.51 (14)
10-44	-15141 / 6640	-209.8	-209.8	0.57 (1)	2.99	12-10	-835 / 854	0.13 (14)
44-11	-16662 / 6973	-209.8	-209.8	0.46 (1)	2.95	41-2	-6834 / 2823	0.59 (1)
1-41	-6508 / 14392	-93.8	-93.8	0.67 (1)	4.82	10-43	-6834 / 2812	0.59 (1)
41-19	-9036 / 20511	-93.8	-93.8	0.95 (1)	4.19	43-44	-819 / 2933	0.00 (1)
19-18	-8074 / 19083	-93.8	-93.8	0.87 (1)	4.43			
18-17	-8074 / 19083	-93.8	-93.8	0.87 (1)	4.43			
17-16	-6264 / 16068	-93.8	-93.8	0.76 (1)	4.93			
16-15	-5713 / 16068	-93.8	-93.8	0.76 (1)	5.11			
15-14	-5713 / 16068	-93.8	-93.8	0.76 (1)	5.11			
14-13	-7524 / 19083	-93.8	-93.8	0.87 (1)	4.57			
13-12	-7524 / 19083	-93.8	-93.8	0.87 (1)	4.57			
12-43	-8489 / 20511	-93.8	-93.8	0.95 (1)	4.31			
43-11	-5971 / 14392	-93.8	-93.8	0.67 (1)	4.99			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {10.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}. INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.8 PSF
DL = 5.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 45.8 PSF

SPACING = 60.0 IN.C.C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED

(80% OF 27.2 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD)
TIMES IMPORTANCE FACTOR EQUALS 23.8 P.S.F.
SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.99")
CALCULATED VERT. DEFL.(LL)= L/900 (0.80")
ALLOWABLE DEFL.(TL)= L/180 (3.98")
CALCULATED VERT. DEFL.(TL)= L/612 (1.17")

CSI: TC=0.92/1.00 (2-4-1), BC=0.95/1.00 (19-41-1),
WB=0.84/1.00 (8-14-3), SSI=0.44/1.00 (4-5-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS=1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MII16	438	302	2547
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (6) (INPUT = 0.90)
JSI METAL= 0.98 (18) (INPUT = 1.00)